BYLAW NO. 1225-21

BEING A BYLAW OF MACKENZIE COUNTY IN THE PROVINCE OF ALBERTA

TO PROVIDE FOR OFF-SITE LEVIES FOR THE PURPOSE OF NORTH SANITARY TRUNK SEWER IN THE HAMLET OF LA CRETE

WHEREAS, pursuant to section 648 of the *Municipal Government Act*, RSA 2000, Chapter M-26, as amended, a municipality has the authority to pass an Off-Site Levy Bylaw; and

WHEREAS, the Council of Mackenzie County, in the province of Alberta, has deemed it necessary to establish an Off-Site Levy Bylaw to pay for the capital costs of new sanitary trunk sewer facilities in the Hamlet of La Crete; and

WHEREAS, the Council of Mackenzie County deems it necessary to require agreements to be entered into with owners of the lands within the boundaries of the Benefitting Areas that are to be subdivided or developed in respect of the payment of the Off-Site Levy; and

WHEREAS, Mackenzie County has prepared a La Crete North Sanitary Trunk Sewer Design Report;

NOW THEREFORE, the Council of Mackenzie County, in the province of Alberta, duly assembled, hereby enacts as follows:

1. **CITATION**

1.1 This bylaw may be cited as the La Crete North Sanitary Off-Site Levy Bylaw and referred to herein as "this Bylaw".

2. **DEFINITIONS**

- 2.1 For the purposes of this Bylaw the following definitions shall apply:
 - a) Act means the *Municipal Government Act*, RSA 2000, Chapter M-26, and amendments thereto;
 - b) Administration means Mackenzie County Administrative Staff;
 - Benefiting Lands means those areas located within Mackenzie County which will benefit from the Off-Site Infrastructure or Improvements subject of this Bylaw;

- d) Council means the Municipal Council of Mackenzie County in the Province of Alberta, as duly elected and defined in the Municipal Government Act, RSA 2000, Chapter M-26 and amendments thereto;
- e) County means the municipal district of Mackenzie County in the Province of Alberta:
- f) Developer means a person or entity who submits a Subdivision or Development Permit Application, pursuant to this Bylaw;
- g) Off-Site Infrastructure *or* Off-Site Improvements means the projects specified in Schedule "A" of this Bylaw for the purposes of sanitary trunk sewer in the Hamlet of La Crete.

3. **APPLICATION**

- 3.1 The total recoverable cost of the Off-Site Infrastructure, subject of this Bylaw is shown in Schedule "A" Section 6.0;
- 3.2 The Off-Site Levy fee is applicable to any Benefiting Lands as shown in Schedule "A" Figures 1 & 2;
- 3.3 The Off-Site Levy fee is charged in accordance with Schedule "A" Executive Summary;
- 3.4 Where it is determined that a development agreement is appropriate for an application for development or subdivision, the developer shall enter into a development agreement with the County and such development agreement shall ensure:
 - a. that provision is made for the payment of the Off-Site Levies as specified in this Bylaw with reasonable interest on the cost of improvements paid for in whole or in part by the municipality as established under the conditions of approval of the development permit for subdivision approval; or
 - b. that provision may be made for the deferring of payment of the Off-Site Levies to a future time certain or uncertain.
- 3.5 In the event that any of the Off-Site Levies imposed by this Bylaw or any other County Bylaw are not paid at the time specified in the development agreement, the County's Chief Administrative Officer is hereby authorized to impose the unpaid sums of money on the lands that are subject of the development agreement, and thereafter collect the same as unpaid taxes in accordance with the provisions of the Act.

4. **SEVERABILITY**

4.1 If at any time any provision of this Bylaw is declared or held to be illegal, invalid, or ultra vires, in whole or in part, then that provision shall not apply and the remainder of this Bylaw shall continue in full force and effect and shall be continued as if it had been enacted without the illegal, invalid, or ultra vires provision.

5. **REPORTING**

5.1 Administration will review the status of Off-Site Levies and provide a report to Council on an annual basis.

6. **ENACTMENT**

- 6.1 Schedule "A" forms part of this bylaw.
- 6.2 This Bylaw shall come into force and effect upon the date of passing of the third and final reading.

READ a first time this 26th day of May, 2021.

READ a second time this 23rd day of June, 2021.

READ a third time and finally passed this 23rd day of June, 2021.

(original signed)
Joshua Knelsen
Reeve

(original signed)

Lenard Racher
Chief Administrative Officer

Schedule "A" La Crete North Sanitary Trunk Sewer Design Report

North Sanitary Trunk Sewer Design Report

Mackenzie County

Hamlet of La Crete

November 16, 2020



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HELIX ENGINEERING LTD.

DISCLAIMER

This Design Report has been prepared by HELIX ENGINEERING LTD for use in preliminary design concepts for the North Sanitary Trunk Sewer for the Hamlet of La Crete in Mackenzie County. The information and data contained herein represent HELIX's best professional judgement in light of the knowledge and information available to HELIX at the time of preparation. This Report and the information and data contained herein are to be treated as confidential and may be used and relied on only by HELIX and its employees. HELIX denies any liability whatsoever to other parties who may obtain access to this document for any injury, loss, or damage suffered by such parties arising from their use of, or reliance upon, this study or any of its contents without the express written consent of HELIX ENGINEERING LTD.



CORPORATE AUTHORIZATION

This document entitled "North Sanitary Trunk Sewer Design Report" was prepared by Helix Engineering Ltd.

APEGA PERMIT

P11731

APEGA 'Permit To Practice' # P11731

Nov 16/20

Randy Glenn, P. Eng

This is a scanned copy of the original.



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EXECUTIVE SUMMARY

Helix Engineering Ltd. has been retained to provide a sanitary sewer servicing strategy for the north and west area of La Crete. The final basin will service 1,442 ha of land with a mix of residential, commercial, and light industrial uses with an allowance for 672 ha of low-pressure sewer flows. The servicing strategy includes three (3) gravity trunk sewers. They are shown on Figure 4 and described as follows:

Phase 1

The Phase 1 basin services 575 ha of a mix of residential and commercial/industrial land including 224 ha of low pressure sewer. It also services the phase 2 and 3 lands. The trunk is located north of 109 Avenue at the Hamlet boundary. It flows west to east along the north boundary of the hamlet to the existing sewage lagoons. The trunk is 2,883m long ranging in size from 450 to 675mm diameter at depths of 3.6 to 9.8m. The trunk drains to a lift station and force main that transfer flows to the lagoon. The resulting peak wet weather flow is 356.5 l/s.

Phase 2

The Phase 2 basin services 610 ha of residential land including 448 ha of low pressure sewer. The trunk is located west of TWP RD 1060, flowing from south to north. The trunk is 1,305m long ranging in size from 375 to 450mm diameter at depths of 5.4 to 7.3m. The trunk drains to a lift station and force main that transfer flows to the phase 1 gravity trunk. The resulting peak wet weather flow is 138.6 l/s. The force main will be 2,223m of 400mm DR11 HDPE pipe.

Phase 3

The Phase 3 services 256 ha of light industrial land located north of the Hamlet. The gravity trunk is 2,200m long with pipes ranging in size from 300mm to 450mm diameter. The pipe depths range from 3.5 to 7.4m.. The peak wet weather flow in the trunk is estimated at 96.5 l/s. This trunk connects to the phase 1 gravity trunk at MH 613.

The estimated cost for the servicing strategy is \$13,787,000 including engineering and contingencies. Based on this cost, levies have been calculated as follows:

Low Pressure \$2,940 /ha

Gravity Area \$15,900 /ha



1.0 GENERAL

The purpose of this report is to consider Sanitary Sewer Servicing Strategy for the north and west areas of La Crete. Final detailed engineering design will be in accordance with the latest Mackenzie County General Municipal Improvement Standards.

2.0 SERVICE AREA

The service area is shown in Figure 1. The lands included area as follows:

West of TWP RD 1060

- East half of 8-106-15-5
- NE5-106-15-5

North of 109 Avenue (TWP RD 1060)

- South half of SW16-106-15-5
- NW16-106-15-5
- East half of 16-106-15-5 and 21-106-15-5
- West half of 15-106-15-5 and 22-106-15-5

The original scope of work included the 3 quarter sections west of TWP RD 1060 and the 3 quarter sections north of and adjacent to 109 Avenue. The alignment of the proposed trunk was changed to allow the additional areas to the north to be serviced within the same trunk system.

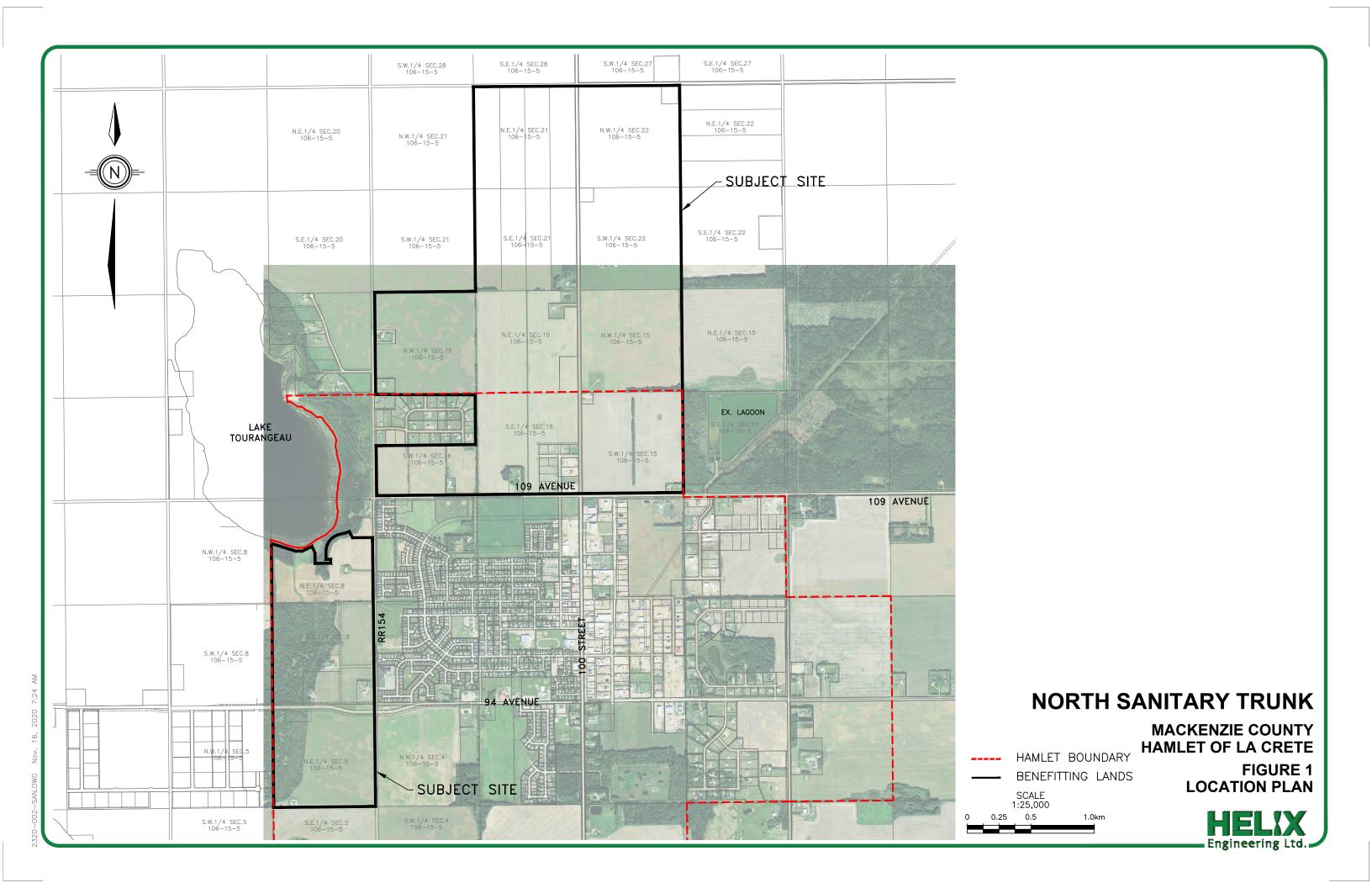
The service area has been discretized into 3 phases. Phase 1 is basin that drains directly into the gravity trunk connecting to the existing lagoon. Phase 2 is the residential area west of RR514 which connects to the phase 1 trunk with a lift station and force main. Phase 3 is the light industrial area to the north, as identified in the Growth Study by 02 Planning in 2020, which connects to the phase 1 trunk by a north expansion gravity trunk.

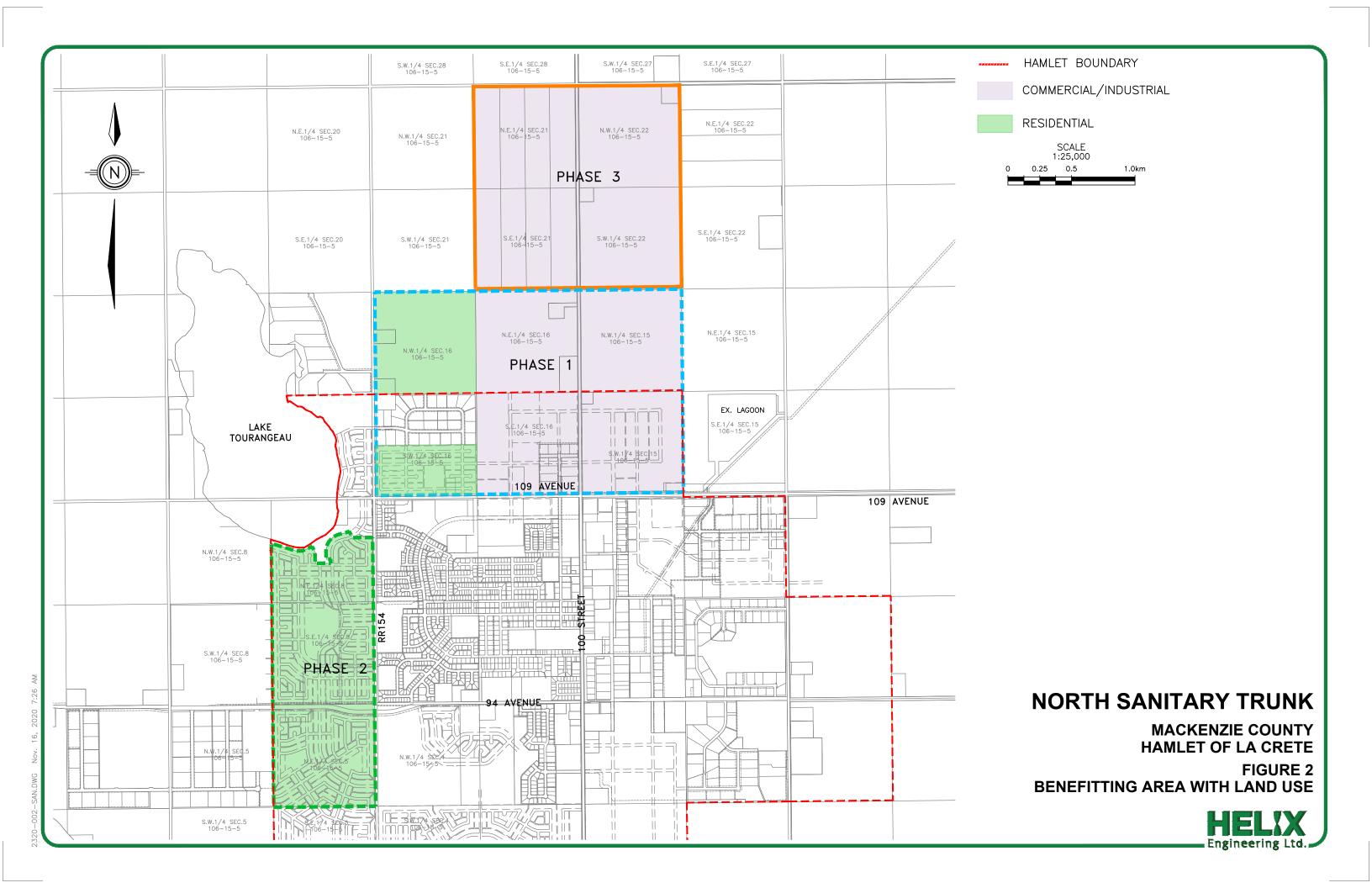
3.0 PROPOSED LAND USE

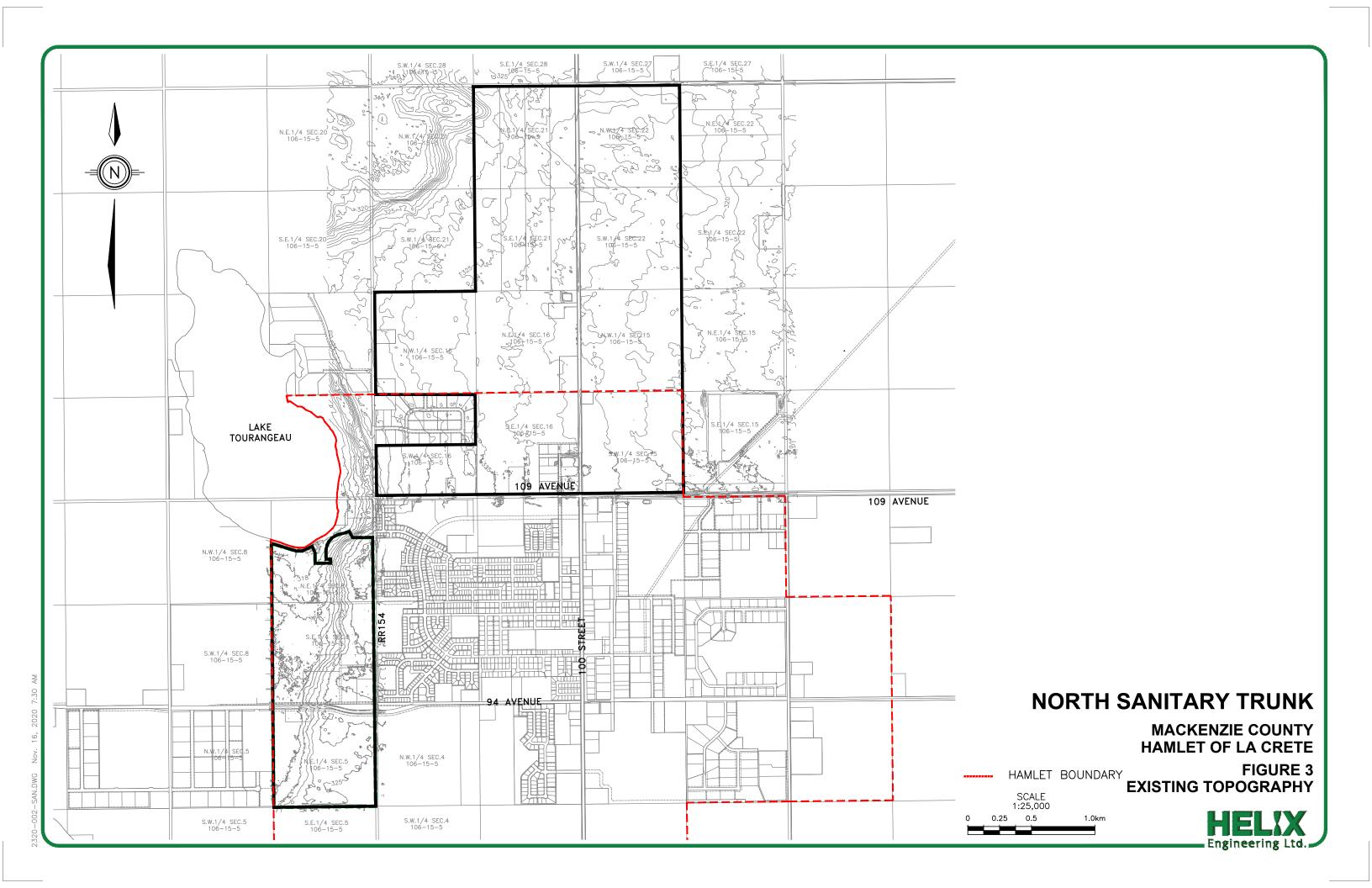
The proposed land use for the basin is shown on Figure 2. The area is predominantly residential with a mix of commercial and industrial. Typically, land uses are based on Area Structure Plans. In the absence of this planning document, the preliminary design is based on the following assumptions:

- Net development land is the gross area less potential Environmental Reserve
- Future arterial road widenings 12.3m 109 Avenue and 10m each side of the Range Roads
- Parks 10% land allocated in residential areas and assumed cash in lieu in industrial areas
- School areas are included in the park allocation
- Residential areas 2% MF and 98% SF
- A School site has been allocated to Phase 2

The existing topography is shown on Figure 3.









The resulting contributing areas are presented in Table 1.

TABLE 1 - LAND USE FOR FL	OW CALCULA	TIONS		
		Phase		Total
Land Use	1	2	3	Total
Gross Area	575.5	610.2	256.0	1,441.7
ER	-	-	-	-
Gross Developable	575.5	610.2	256.0	1,441.7
Road Widenings	4.7	2.7	1.9	9.3
Stormwater Management	12.1	-	8.1	20.2
Parks (net of Schools)	9.6	4.3	-	13.9
Subtotal	26.4	7.0	10.0	43.4
Net Developable	549.1	603.2	246.0	1,398.4
LPS	224.0	448.0	-	672.0
Residential	81.4	146.2	-	227.7
Schools	-	9.0	-	9.0
Com/Ind	243.7	-	246.0	489.7
Total	549.2	603.2	246.0	1,398.4
Land areas in ha. Phase 3 is t	he industrial exp	oansion area.		

4.0 DESIGN CRITERIA

The design criteria used in the preliminary design is in accordance with the County Mackenzie County General Municipal Improvement Standards dated July 2014. Where gaps occurred, standards were used from other municipalities. The preliminary design is based on the following criteria:

- Residential Flow Generation 350 I/p/d (equates to 0.00405 I/s/p)
- Single Family Density of 35 p/ha
- Multi-family Density of 105 p/ha
- Commercial / Industrial / Institutional 3,000 l/ha/d (equates to 0.035 l/s/ha)
- Low Pressure Sewer (LPS) servicing 4.16 l/s/ quarter section (based on 30 lots per quarter)
- Peaking Factor Residential $PF = 1 + \frac{14}{4 + (P/1000)^{0.5}}$ 2.5<PF<5
- Peaking Factor Ind/Com/Inst $PF = 10xQave^{-0.45}$ 2.5<PF<5
- Inflow and Infiltration 17,000 l/s/ha (equates to 0.20 l/s/ha); peaking factor does not apply
- Force main maximum velocity 2.0 m/s
- Force main roughness co-efficient 140
- LPS is not subject to peaking factors or inflow/infiltration
- Maximum manhole spacing 150m



In addition to the criteria listed above, Mackenzie County and Helix Engineering Ltd, in consultation with Aquatera Utilities, have agreed on the following:

- Historical data for light industrial areas in the Clairmont area of the Aquatera service area indicate that the generated flows are significantly less than the industrial standard would predict. It is expected that development would be of a similar nature in this area of La Crete. As a result, the flow generation rate has been lowered from the County's standard of 17,000 l/ha/day to 3,000 l/ha/day.
- The LPS flows have the potential to deteriorate concrete manholes when entering the gravity system.
 Manholes can be lined to protect against corrosion. The County inspected manholes from the
 connection point of the existing LPS systems and found some deterioration in the first couple of
 manholes only. Thus, this report includes lining of the first three manholes, the connection manhole
 and two downstream.
- The use of Vortex systems where LPS or force mains connect to the gravity trunk should be investigated. When the turnover in the pressure pipe takes longer than a day, the use of a vortex may be warranted. This will be the case in the early stages of development when there are minimal flows in the system and the pipes are sized for the ultimate. The Vortex system will reduce odors and corrosion. Vortex systems have been included at the LPS connection, but not at the force main connection at the lagoon.

Based on the design criteria, the peak wet weather flows have been calculated for each inflow manhole to be used in sizing the trunk sewer. The resulting flows are presented in Table 2.

		TABLE	2 - DESI	GN FLOV	vs									
		F	hase - S	tand Alon	е		System -	Total						
Land Use 1 2 3 System Total														
LPS	14.6	9.5%	29.1	21.0%	-	0.0%	43.7	12.3%						
Residential & Schools	57.7	37.6%	109.5	79.0%	-	0.0%	167.2	46.9%						
Com/Ind	81.3	52.9%	-	0.0%	81.9	100.0%	145.7	40.9%						
Total	153.6	100.0%	138.6	100.0%	81.9	100.0%	356.6	100.0%						
Flows are Peak Wet Weath	er (l/s)		·					·						



5.0 TRUNK DESIGN

The land within the basin slopes from south to north and west to the east. The general design concept is for three gravity trunks and 2 lift stations to convey flows from the west to the east, to the existing sewage lagoon.

The Phase 1 gravity trunk will flow into a sanitary lift station. The lift station will be located at the existing sewage lagoon and will pump flow to the sewage lagoon in a short force main.

The Phase 2 gravity trunk will collect flows in the west basin and connect to a lift station. The lift station will pump the flows into the top of the Phase 1 trunk thru a force main.

The Phase 3 gravity trunk will collect flows in the north basin and connect to the Phase 1 trunk.

Detailed flow calculations are included in Appendix A.

The alignments were selected in consultation with County staff. It was also agreed that the trunk designs would allow for LPS flows, 3.5 quarters into Phase 1 and 7 quarters into Phase 2. Phase 3 does not have an LPS contribution.

Design of the trunk sewer considers the depth required for the lateral connections servicing the basin. Details of each phase are as follows:

Phase 1

The Phase 1 gravity trunk sewer is 2,883m long with pipe sizes ranging from 450mm to 675mm diameter. The peak dry weather flow is 242.3 l/s and the peak wet weather flow is 356.5 l/s. These flows include an allowance for 14.6 l/s of LPS inflow, representing 224 ha of development, of which approximately 26.2 ha (1.7 l/s) is existing. The depth of the trunk ranges from 3.6m to 9.8m. The trunk connects to a lift station at the existing lagoon. The lift station will convey flows from the trunk to the lagoon. Ultimately, the force main will be 50m of 600mm HDPE with a pressure of 20 psi. Given the short distance for the force main, the sizing of pumps and force main should be staged as warranted by development within the basin.

Phase 2

The Phase 2 gravity trunk sewer is 1,305m long with pipe sizes ranging from 375mm to 450mm diameter. The peak dry weather flow is 107.6 l/s and the peak wet weather flow is 138.6 l/s. These flows include an allowance for 29.1 l/s of LPS inflow representing 448 ha of development, of which approximately 48.9 ha (3.2 l/s) is existing. The depth of the trunk ranges from 5.4 to 7.3m. The trunk connects to a lift station at the north end. The lift station will convey flows from the trunk to the Phase 1 trunk. Ultimately, the force main will be 2,223m of 400mm HDPE with a pressure of 35 psi. Initial pumps should be sized for flow rate of 59 l/s at approximately 23 psi to achieve a velocity of 0.6m/s. The pumps running for 1 hour per day will turn over the volume in the pipe each day.

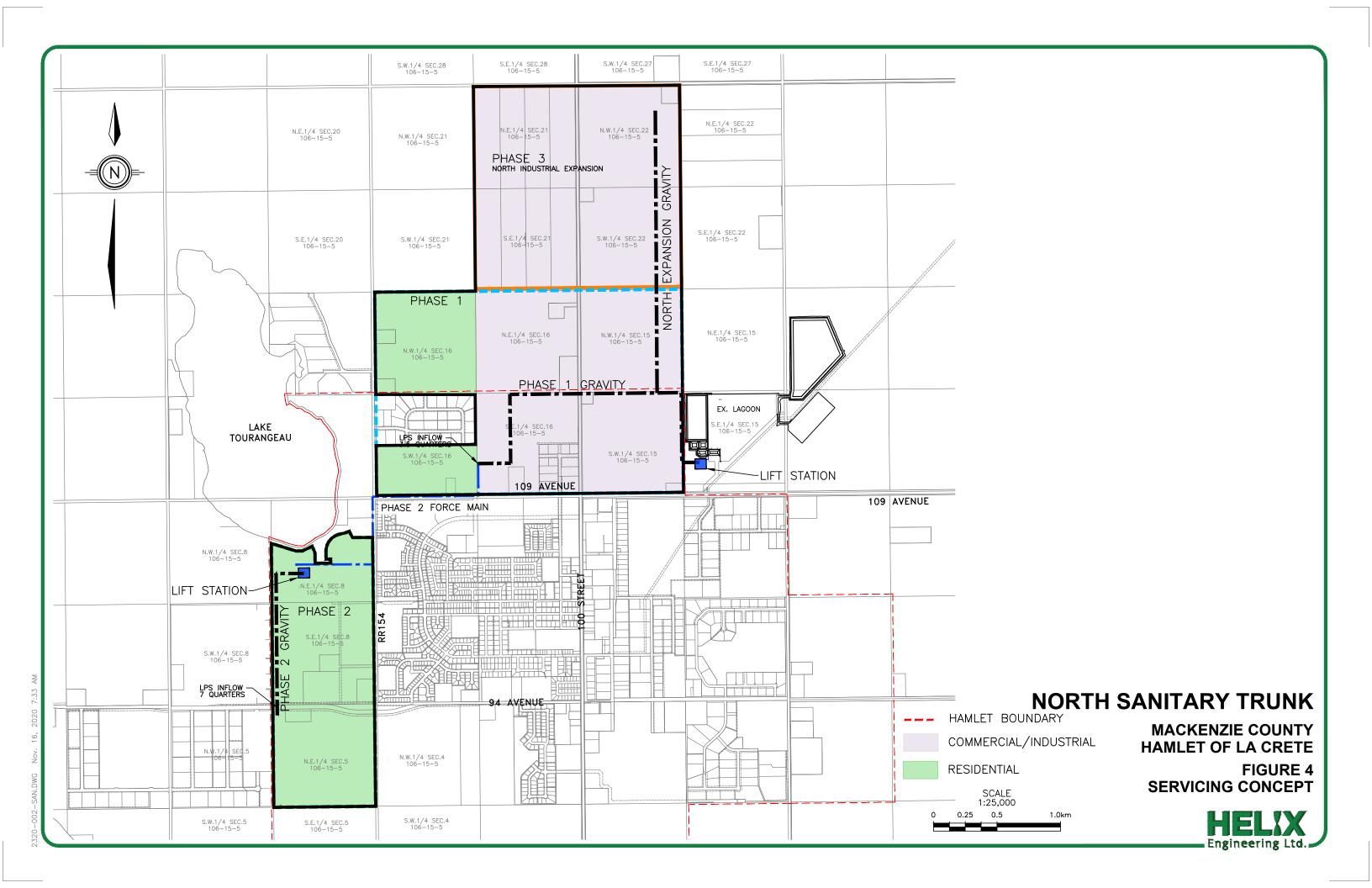
Phase 3

A conceptual design has been done for the phase 3 gravity trunk to provide construction cost estimates and determine the depth required at the Phase 1 manhole to allow the connection. The phase 3 gravity trunk is 2,200m long with pipe sizes ranging from 300mm to 450mm diameter. The peak dry weather flow is 36.5 l/s and the peak wet weather flow is 96.5 l/s. The flows result from portions of the Phase 2 basin connecting to



the south end of this trunk for efficient servicing. The stand-alone peak wet weather flow is 81.9 l/s. These flows do not include any allowance for LPS flows. The depth of the trunk ranges from 3.6m to 7.4m. The trunk connects to the Phase 1 gravity trunk at MH#613.

The servicing concept is shown on Figure 4.





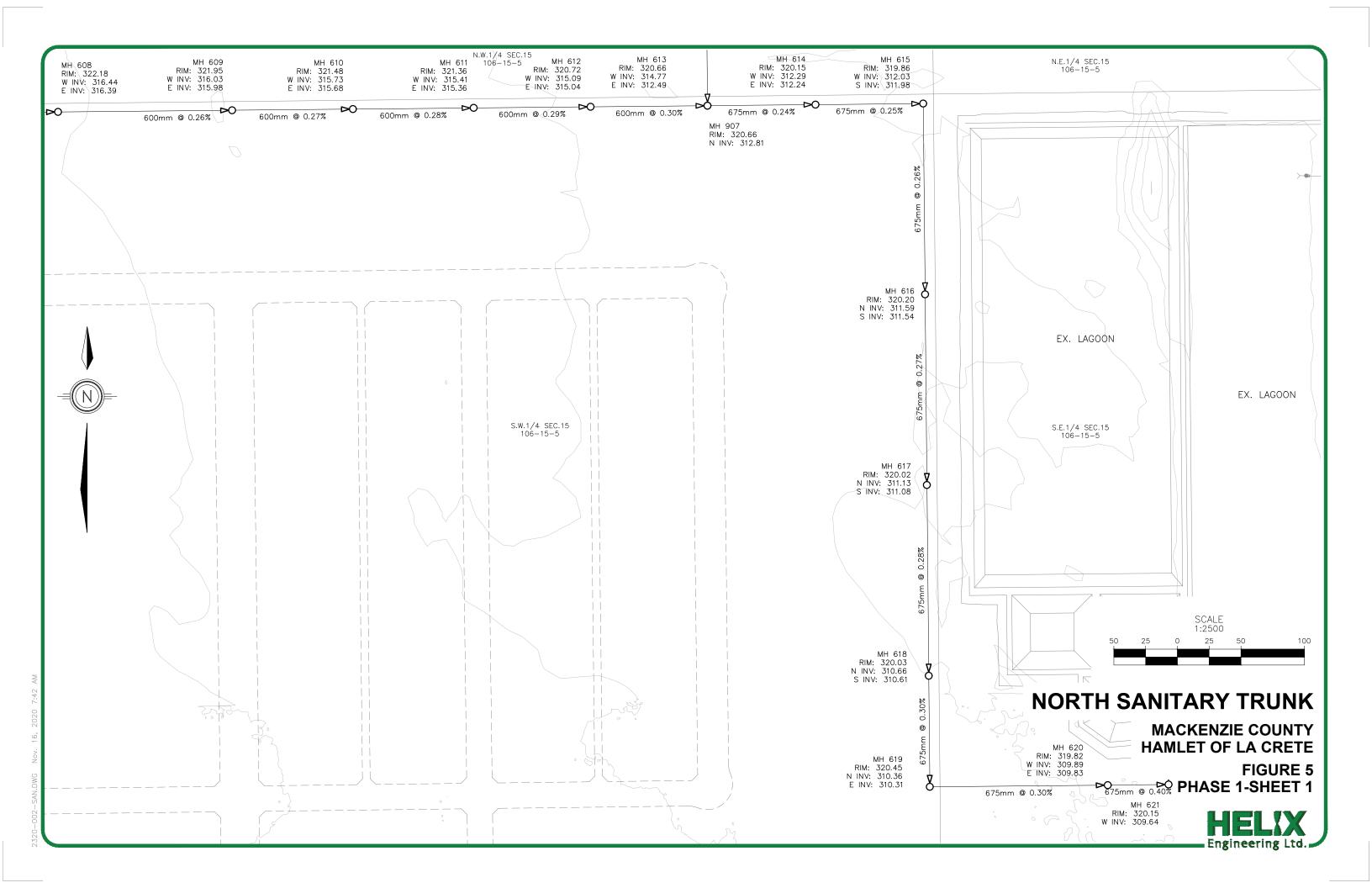
6.0 CONSTRUCTION COST ESTIMATES

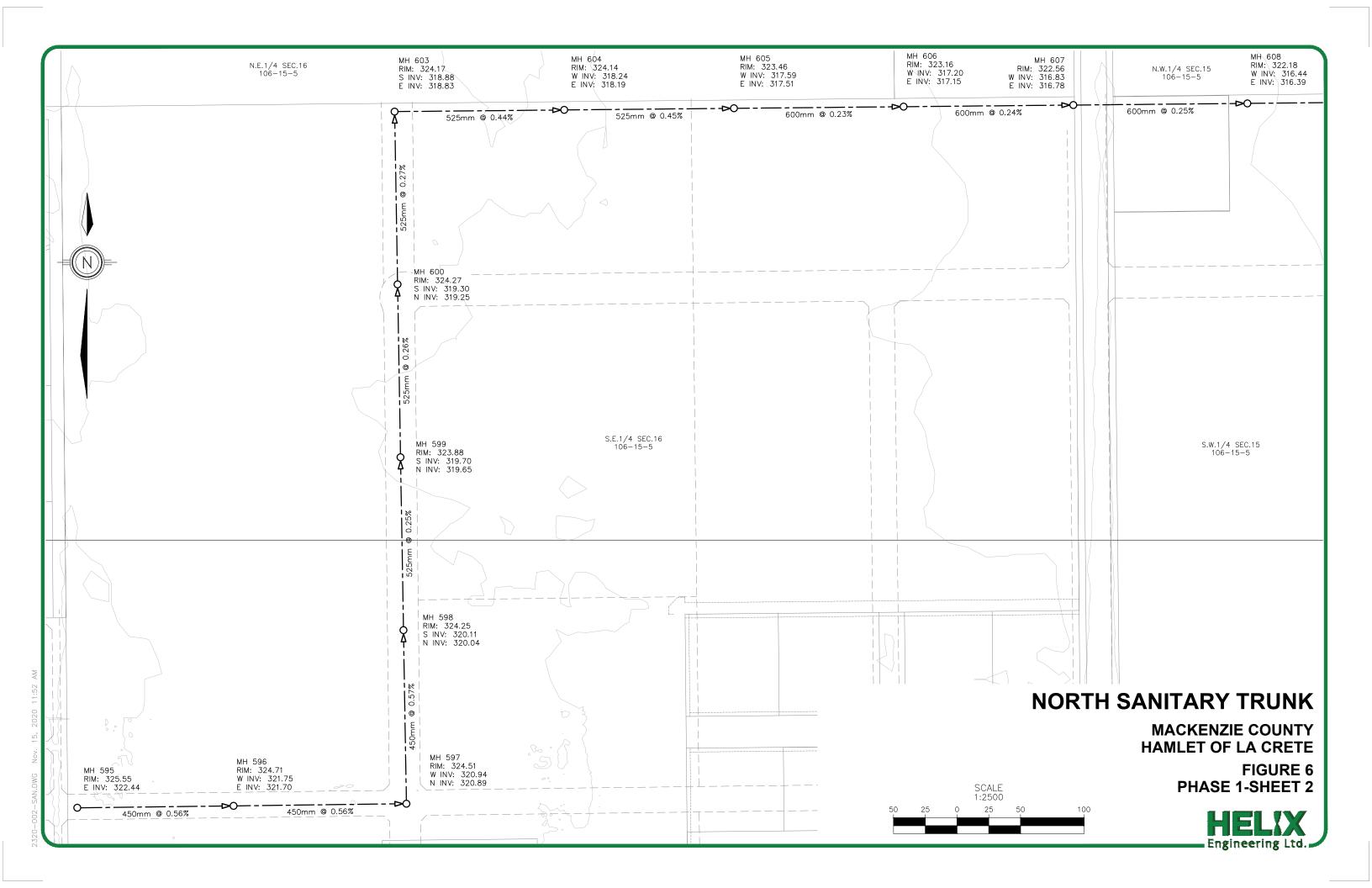
The construction cost for the servicing concept has been estimated based on the preliminary design of the system. This includes the gravity trunks, lift stations and the force mains as illustrated on Figures 5 to 8. The detailed cost estimates are included in Appendix B. Estimates include the following:

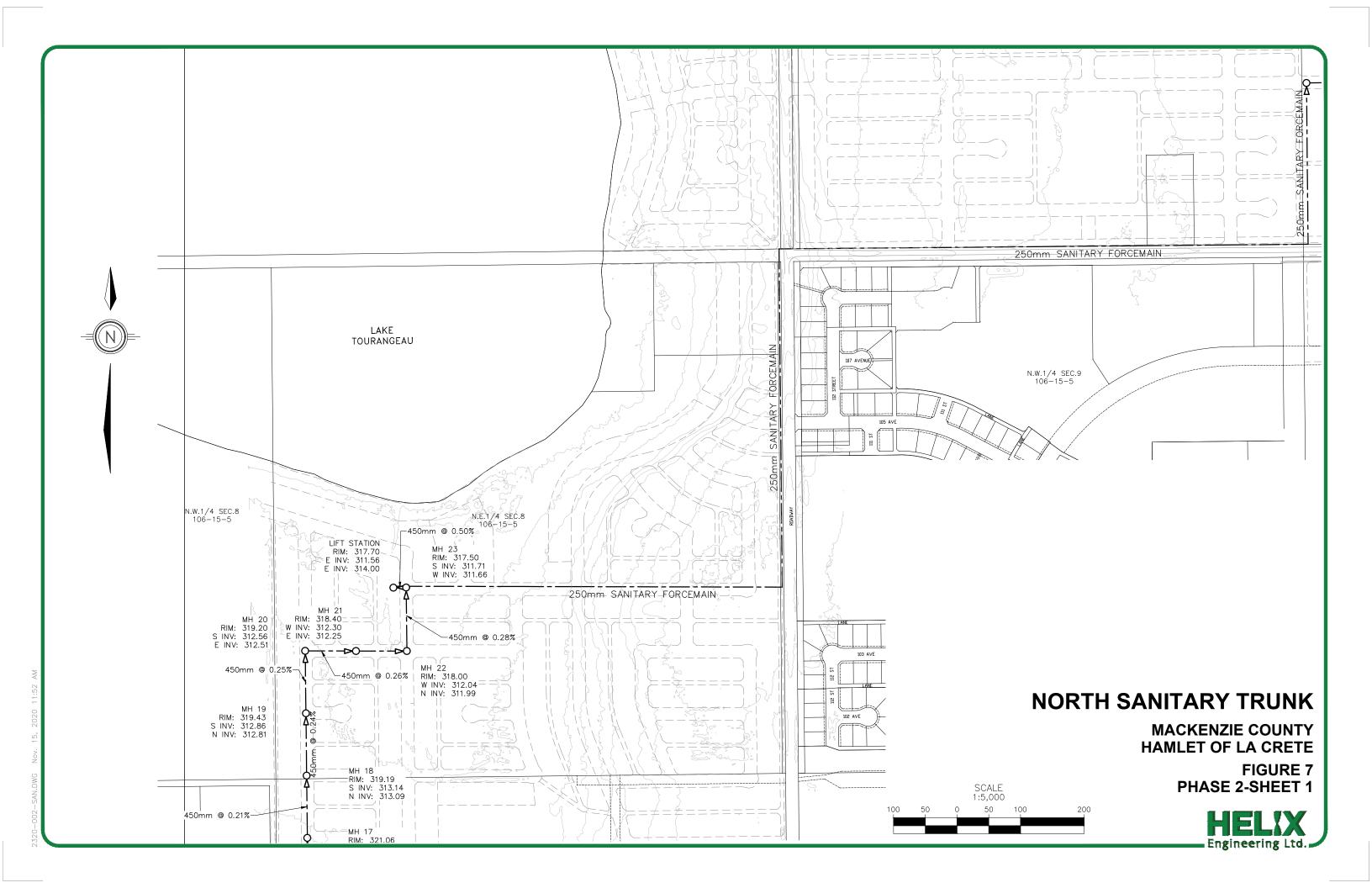
- Cost of the servicing study
- 10% for Engineering
- 20% for Contingencies (30% for Phase 3)
- Allowance for lining of three upper manholes to facilitate the LPS connection
- Trenchless construction of the force main

The construction costs are summarized in Table 3.

TABLE 3 - ESTIMATED COST	ΓS			
		Phase		Total*
	1	2	3*	Total
Gravity Trunk	2,753,000	1,057,000	1,254,000	5,064,000
Lift Station / Force Main	2,094,000	3,172,000	-	5,266,000
Subtotal	4,847,000	4,229,000	1,254,000	10,330,000
Contingencies 20%*	970,000	846,000	377,000	2,193,000
Engineering 10%	485,000	423,000	126,000	1,034,000
Subtotal	6,302,000	5,498,000	1,757,000	13,557,000
Trunk Sewer MH Lining	27,000	43,000	-	70,000
Design Report	60,000	60,000	40,000	160,000
Project Total	6,389,000	5,601,000	1,797,000	13,787,000
* Phase 3 Contingency is 30%				







MACKENZIE COUNTY VILLAGE OF LA CRETE FIGURE 8

SCALE 1:5,000 50

100

200

PHASE 2-SHEET 2







7.0 BASIN LEVIES

The cost to service the basin will be charged back to the benefitting lands as a development levy. Separate levy rates are presented for the future LPS system and the gravity trunk servicing area. Costs have been apportioned based on the portion of the peak wet weather flow as shown in Table 3. The resulting cost allocations and development levies are shown Table 4.

TABLE 4 - DEVELOPMENT L	EVIES		
	Cost	Area (ha)	Levy
Residential	6,432,000	258.2	24,910
Com/Ind	5,604,000	499.0	11,240
Gravity Levy	12,036,000	757.2	15,900
LPS	1,681,000		
Extra Cost for MH Lining	70,000		
LPS Levy	1,751,000	596.9	2,940
Total	13,787,000		
Benefiting Area		1,354.1	
Cost amounts are rounded to	nearest \$1,000.		
Levies are arounded to neares	t \$10.		



Appendix A

Design Flow Calculations

Mckenzie County
Sanitary Sewer Flows
LaCrete North Trunk Sewer

Phase 1 I/I = 0.2 I/s/ha Density: Phase 2 and Phase 3 inflows included. 2320-002 units per ha 10 350 Vp/d Res 3,000 l/ha/d Commercial Flow Dry Weath Peak System Design Pipe Data DNS Population Manhole / Basin Sag Accum Manhole Sags Sewage Generation Capacity Diameter Slope Ups Inv Length Dns Inv Pipe Drop MH Drop Curved Depth Accum. Area Accum. Density Accum I/I -Area Total Dns (ha) (people/ha) (l/s) (l/s) (l/s) (mm) (m/m) Ups (people) (rate) (l/s) (l/s) (l/s) 0 25.82 SF 595 596 West 25.82 35 904 0.00405 Vs/p 3.66 0.53 MF 0.53 105 0.00405 Vs/p 0.22 Residential 26.35 959 3.89 14.81 5.27 20.08 23.24 8.60 Com/Ind 8.60 0.00 0.03500 l/s/ha 0.3 0.20000 l/s/ha 320.8 School Non-Residential 1.51 1.72 3.23 3.73 595 Ups MH 596 Dns MH Direct Flow 138.6l/s From Phase 2 + 3 quarters of LPS 153.18 153.18 177.30 450 0.0056 320.71 0.689 34.95 34.95 204.27 213.24 324.70 3.539 0.0012 596 597 West 0 SF 25.82 35 904 0.00405 l/s/p 3.66 0.22 ME 0.53 105 55 0.00405 Vs/p 3.89 14.81 5.27 20.08 Residential 26.35 23.24 Com/Inc 0.03500 l/s/ha School 0.00 0.20000 l/s/ha 597 Dns MH Non-Residential 5.00 0.3 1.51 3.23 3.73 596 Ups MH 8.60 Direct Flow 153.18 177.30 136 319.90 0.762 0.05 n 3.59 324.50 4.150 Total 34.95 204.27 213.24 450 0.0056 0.0012 597 598 0 RG 25.82 0.00405 l/s/p 35 904 3.66 MF 0.53 105 0.00405 l/s/p 0.22 14.81 5.27 20.08 23.24 Residential 26.35 3.89 7.40 Com/Ind 0.03500 l/s/ha 16.00 0.6 0.20000 Vs/ha Non-Residential 16.00 0.6 2.80 3.20 6.00 6.94 597 Ups MH 598 Dns MH Add N half of SW 16 LPS Direct Flow 153.18 42.35 207.48 215.14 450 0.0057 136 319.07 0.775 0.075 n 324.20 4.676 7.40 598 599 0 RG 25.82 35 904 0.00405 l/s/n 3.66 0.22 0.53 105 55 0.00405 Vs/p 23.24 Residential 8.67 Com/Ind 24.67 0.03500 l/s/ha 0.9 0.20000 l/s/ha School 0.00 Non-Residential 0.9 4.32 4.93 9.25 10.71 598 Ups MH 599 Dns MH Add N half of SW 16 LPS 153.18 318.66 0.340 8.67 51.02 211.24 214.92 525 0.0025 323.90 4.716 0.0010 3.66 0 25.82 35 0.00405 l/s/p ME 0.53 105 55 0.00405 l/s/p 0.22 14.81 5.27 20.08 23.24 Residential 26 35 3.89 2.59 Com/Ind 27.26 0.03500 Vs/ha School 0.00 0.20000 l/s/ha Non-Residential 27.26 5.00 1.0 4.77 5.45 10.22 11.83 599 Ups MH 600 Dns MH 153.18 2.59 53.61 212.37 219.18 525 0.0026 600 603 0 RG 25.82 35 0.00405 Vs/p 3.66 105 0.00405 Vs/p 0.22 0.53 55 Residential 14.81 5.27 20.08 23.24 Com/Ind 0.03500 l/s/ha 27.26 1.0 School 0.00 0.20000 l/s/ha 11.83 177.30 Non-Residential 4.77 5.45 10.22 600 Ups MH 603 Dns MH 153.18 Direct Flow 53.61 317.84 0.367 212.37 324.17 5.806 0.0010

2396-002 - North Trunk - Design Report - 11/16/2020

Phase 1 2320-002			Phase 2 and P	Phase 3	inflows included.	Density: units per		10		350 l/p/d Res 3,000 l/ha/d Comm			VI = 0.2 Vs/H	ha							
Manhole / Basin Ups Dns			Sag Ac Manhole Sa	ccum. ags	Area Zoning (ha)	Accum. Area	Populati Density (people/	Ac	cum. iople)	Sewage Generation (rate)	PF	Flow Dry Weath Pea (I/s) (I/s)	k I/I -Area Tot (I/s) (I/s)		Design (l/s)	System Design Capacity Diameter Slope (Vs) (mm) (m/m)	Rim Pipe Data Ups Inv	_ength Dns Inv Pipe Drop N	NS UPS IH Drop Curved Depth	DNS Rim	Depth
603	604		0	(54.00 RG 1.10 MF		0.82 .63	35 105	2,794 171			11.32 0.69									
	Re	esidential		(64.69 Com/l		.45		2,965	0.03500 Vs/ha	3.45	12.01 3.2	41.40 16.29	57.6	9 66.77						
					- School		.95			0.20000 Vs/ha		3.2									
		on-Residential irect Flow				91	.95				5.00	3.2	16.09 18.39	34.4 153.1			603 Ups MH	604 Dns MH			
												<u> </u>		153.1							
	To	otal			119.79	173.	40								283.97	285.12 525 0.0044 Min. 0.0010	324.17 317.79	133.5 317.20 0.587	0.05 n 5.8	32 32	4.14 6.414
604	605		0	(9.82	35	2,794	0.00405 l/s/p		11.32									
	Re	esidential		(- MF		.63 .45	105	171 2,965		3.45	0.69 12.01	41.40 16.29	57.6	9 66.77						
	-				- Com/l	nd 91	.95		_,,,,,,	0.03500 Vs/ha	0.10	3.2				-					
	No	on-Residential			- School		.95			0.20000 l/s/ha	5.00	3.2	16.09 18.39	34.4	8 39.91		604 Ups MH	605 Dns MH			
	Dii	irect Flow												153.1							
	То	otal			-	173.	40								283.97	288.35 525 0.0045	324.14 318.21	133.5 317.61 0.601	0.075 n 5.4	1 32	3.44 5.310
605	606		0		. RG	70	0.82	35	2,794	0.00405 Vs/p		11.32				Min. 0.0010					
605	606		U	,	- KG	1	.63	105	2,794	0.00405 Vs/p		0.69									
	Re	esidential		(17.24 Com/l		.45		2,965	0.03500 Vs/ha	3.45	12.01 3.8	41.40 16.29	57.6	9 66.77						
					- Schoo	1 0	0.00			0.20000 Vs/ha		-									
		on-Residential irect Flow				109).19				5.00	3.8	19.11 21.84	40.9 153.1			605 Ups MH	606 Dns MH			
														133.1							
	То	otal			17.24	190.	64								291.45	294.32 600 0.0023 Min. 0.0010	323.44 317.79	133.5 317.48 0.307	0.05 n 5.0	1 <mark>5</mark> 32	3.16 5.078
606	607		0	(9.82	35	2,794			11.32									
	Re	esidential		(- MF		.63 .45	105	171 2,965		3.45	0.69 12.01	41.40 16.29	57.6	9 66.77						
					- Com/l	nd 109	9.19		,,,,,	0.03500 l/s/ha		3.8				=					
607 is the range r		on-Residential			- School		0.00 0.19			0.20000 l/s/ha	5.00	3.8	19.11 21.84	40.9	5 47.39		606 Ups MH	607 Dns MH			
	Di	irect Flow												153.1	B 177.30	-					
	То	otal			-	190.	64								291.45	300.65 600 0.0024	323.16 317.15	133.5 316.83 0.320	0.05 n 5.4	1 32	2.64 5.209
607	608		0) - RG	70	0.82	35	2,794	0.00405 Vs/p		11.32				Min. 0.0010					
607			Ü	,	- MF	1	.63	105	171	0.00405 Vs/p		0.69									
	Re	esidential		(12.50 Com/l		.69		2,965	0.03500 Vs/ha	3.45	12.01 4.3	41.40 16.29	57.6	9 66.77						
607 is the range r	road				- School	1 0	0.00			0.20000 Vs/ha		-				_					
		on-Residential irect Flow				121	.69				5.00	4.3	21.30 24.34	45.6 153.1			607 Ups MH	608 Dns MH			
		ntal			12.50	203.									296.88		322.64 316.78	137 316.44 0.343	0.05 n 5.2		2.09 5.052
	10	ла			12.50	203.	.14								290.00	Min. 0.0010	322.04 310.76	137 310.44 0.343	0.05 11 5.2	32	2.09 5.052
608	609		0	(- RG - MF		.82 .63	35 105	2,794 171	0.00405 Vs/p 0.00405 Vs/p		11.32 0.69									
	Re	esidential		()	81	.45	105	2,965		3.45	12.01	41.40 16.29	57.6	9 66.77						
					- Com/l		.69).00			0.03500 Vs/ha 0.20000 Vs/ha		4.3									
	No	on-Residential			- 30100		.69			0.20000 VS/IIa	5.00	4.3	21.30 24.34	45.6			608 Ups MH	609 Dns MH			
	Dii	irect Flow												153.1	B 177.30						
	To	otal				203.	14								296.88		322.09 316.39	137 316.03 0.356	0.05 n 5.1	0 32	1.91 5.278
609	610		0	() - RG - MF	79	0.82 .63	35 105	2,794 171	0.00405 Vs/p 0.00405 Vs/p		11.32 0.69				Min. 0.0010					
	Re	esidential		()	81	.45	100	2,965		3.45	12.01	41.40 16.29	57.6	9 66.77						
					30.75 Com/l - School		2.44			0.03500 Vs/ha 0.20000 Vs/ha		5.3									
		on-Residential			00100	152	2.44			5.20000 ¥3/1ld	4.71	5.3	25.12 30.49	55.6			609 Ups MH	610 Dns MH			
		irect Flow												153.1							
-	То	otal			30.75	233.	89								308.42	318.89 600 0.0027 Min. 0.0010	321.91 315.98	95 315.73 0.257	0.05 n 5 .3	32	1.37 5.044

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ase 1 0-002		Phase 2 a	and Phase	3 inflows in	ncluded.	Density: units per ha	ā	10		350 l/p/d Res 3,000 l/ha/d Commer	rcial		I/I = 0.2 I	/s/ha														
hole / Basin Dns		Sag Manhole	Accum. Sags	Area (ha)	Zoning	Accum. Area	Population Density (people/ha)	Accu		Sewage Generation (rate)	PF	Flow Dry Weath Pea (I/s) (I/s)		Total I/s)	Design (l/s)		System Des Capacity (l/s)		Slope (m/m)	UPS Rim	Pipe Data Ups Inv Leng	yth Dns	s Inv Pipe	DNS Drop MH Drop	Curved D		NS im	Depth
610 6	611		0	0 -	RG MF	79.8 1.6		35 105	2,794 171	0.00405 Vs/p 0.00405 Vs/p		11.32 0.69																
	Residential			0		81.4	15	100	2,965	· ·	3.4	5 12.01	41.40 16.29		57.69	66.77												
					Com/Ind School	152.4				0.03500 l/s/ha 0.20000 l/s/ha		5.3																
	Non-Residential				SCHOOL	152.4				0.20000 Vs/na	4.7		25.12 30.49		55.60	64.36					610 Ups MH		611 Dns	мн				
	Direct Flow					.,								1	53.18	177.30												
	Total					233.89	9									308.42	324.74	4 600	0.0028	32	1.37 315.68	95	315.41	0.266 0.0	5 n	5.09	321.42	5.4
																		Min.	0.0010						-			
611 6	312		0	0 -		79.8		35	2,794	0.00405 Vs/p		11.32																
	Residential			0	MF	1.6 81.4		105	171 2,965	0.00405 Vs/p	2.	0.69 5 12.01	41.40 16.29		57.69	66.77												
	residential			-	Com/Ind				2,303	0.03500 l/s/ha	J	5.3	41.40 10.23		37.03	00.77												
					School	0.0				0.20000 Vs/ha		-																
	Non-Residential					152.4	14				4.1		25.12 30.49		55.60 53.18	64.36 177.30					611 Ups MH		612 Dns	MH				
	Direct Flow											-		1;	53.18	177.30												
	Total					233.89	9									308.42	330.49				1.42 315.36	92	315.09	0.267 0.0	5 n	5.46	320.71	5.0
			0	0 -		70.0		35	0.704	0.00405.1/ /		****						Min.	0.0010									
612 6	613		U	0 -		79.8 1.6		105	2,794 171	0.00405 Vs/p 0.00405 Vs/p		11.32 0.69																
	Residential			0		81.4		100	2,965		3.4		41.40 16.29		57.69	66.77												
	<u> </u>			-	Com/Ind					0.03500 l/s/ha		5.3																
	Non-Residential				School	0.0 152.4				0.20000 Vs/ha	4.7	1 5.3	25.12 30.49		55.60	64.36					612 Ups MH		613 Dns	ML				
	Direct Flow					102.4					7.	1 3.3	20.12 30.40		53.18	177.30					012 Ops Wil 1		013 0113	IVII I				
	Total			-		233.89	9									308.42	336.14	4 600 Min.	0.0030		0.71 315.04	92	314.77	0.276 0.07	5 n	5.07	320.64	5.27
613 6	614		0	0 -	RG	79.8	32	35	2,794	0.00405 Vs/p		11.32						IVIII I.	0.0010									
					MF	1.6	3	105	171			0.69																
	Residential			0	48 Com/Ind	81.4 459.9			2,965	0.03500 Vs/ha	3.4	5 12.01 16.1	41.40 16.29		57.69	66.77												
					School	0.0				0.20000 Vs/ha		-																
	Non-Residential					459.9					2.8	6 16.1	46.10 91.98		38.08	159.82					613 Ups MH		614 Dns	MH				
	Direct Flow													15	53.18	177.30												
	Total			307.4	48	541.37	7									403.88	411.59	9 675	5 0.0024	32	0.64 312.50	85	312.30	0.204 0.0	5 n	7.46	320.11	7.13
																		Min.	0.0010									
614 6	615		0	0 -		79.8 1.6		35 105	2,794 171	0.00405 Vs/p 0.00405 Vs/p		11.32 0.69									312.81 Inv o Calc 450 Gravity							
	Residential			0	IVIF	81.4		105	2,965		3.4		41.40 16.29		57.69	66.77					312.585 max							
						459.9)2		_,	0.03500 Vs/ha		16.1																
					School	0.0				0.20000 l/s/ha		-																
	Non-Residential Direct Flow					459.9)2				2.8	6 16.1	46.10 91.98		38.08 53.18	159.82 177.30					614 Ups MH		615 Dns	MH				
	DIRECT FIOM														JJ. 10	111.30												
						541.37										403.88							312.03					7.202

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Phase 1 2320-002			Phase 2 a	and Phase	3 inflows in	ncluded.	Density: units per ha		10	35 3,0	i0 l/p/d Res 000 l/ha/d Comme			1/1 =	0.2 l/s/ha											
Manhole / Basin Ups Dns			Sag Manhole	Accum. Sags	Area (ha)	Zoning	Accum. E	Population Density (people/ha)	Accum. (people)		ewage Generation ate)	PF	Flow Ory Weath Pea I/s) (I/s)		-Area Total s) (l/s)	Design (I/s)		System Design Capacity Dia (Vs) (mr	meter Slope n) (m/m)	UPS Rim	Pipe Data Ups Inv Length				ONS Rim De	epth
615	616			0	0 -		79.82			2,794	0.00405 l/s/p		11.32													
	F	Residential			0	MF	1.63 81.45	1	105	171 2,965	0.00405 Vs/p	3.45		41.40	16.29	57.69	66.77									
	_				1	Com/Ind School	459.92 0.00				0.03500 Vs/ha 0.20000 Vs/ha		16.1													
		Non-Residential				OCHOOL	459.92				0.20000 ¥3/1la	2.86	16.1	46.10	91.98	138.08	159.82			6	15 Ups MH	616 Dns MH				
		Direct Flow														153.18	177.30									
-		Fotal			-		541.37										403.88	428.40 Min	675 0.0026		91 311.98	150 311.59 0.390	0.05 n	7.25	320.25	7.981
616	617			0	0 -		79.82		35	2,794	0.00405 l/s/p		11.32					IVIIII	. 0.0010							
	F	Residential			0	MF	1.63 81.45	1	105	171 2,965	0.00405 l/s/p	3.45	0.69 12.01	41.40	16.29	57.69	66.77									
	-				-	Com/Ind	459.92			_,,	0.03500 Vs/ha	0.10	16.1													
	ī	Non-Residential				School	0.00 459.92				0.20000 l/s/ha	2.86	16.1	46.10	91.98	138.08	159.82			6	16 Ups MH	617 Dns MH				
		Direct Flow														153.18	177.30				,-					
	-	Fotal					541.37										403.88	436.56	675 0.0027		25 311.54	150 311.14 0.405	0.05 n	8.03	320.15	8.336
617	618			0	0 -	RG	79.82		35	2,794	0.00405 Vs/p		11.32					Min	. 0.0010							
					-		1.63		105	171	0.00405 l/s/p		0.69													
	<u>.</u>	Residential			0	Com/Ind	81.45 459.92			2,965	0.03500 l/s/ha	3.45	12.01 16.1	41.40	16.29	57.69	66.77									
		Non-Residential			-	School	0.00 459.92				0.20000 l/s/ha	0.00	-	46.10	04.00	138.08	159.82				47 H MH	618 Dns MH				
		Direct Flow					459.92					2.86	16.1	46.10	91.98	153.18	177.30			ь	17 Ups MH	618 DNS MH				
	-	Fotal					541.37										403.88	444.57	675 0.0028	320	15 311.09	150 310.67 0.420	0.05 n	8.39	320.06	8.717
																		Min								
618	619		1	0	0 -		79.82 1.63		35 : 105	2,794 171	0.00405 Vs/p 0.00405 Vs/p		11.32 0.69													
	F	Residential			0	C/II	81.45			2,965		3.45	12.01 16.1	41.40	16.29	57.69	66.77									
					- 1	Com/Ind School	459.92 0.00				0.03500 l/s/ha 0.20000 l/s/ha		-													
		Non-Residential Direct Flow					459.92					2.86	16.1	46.10	91.98	138.08 153.18	159.82 177.30			6	18 Ups MH	619 Dns MH				
																100.10										
		Fotal			-		541.37										403.88	452.44 Min	675 0.0029 . 0.0010		06 310.62	88 310.36 0.255	0.05 n	8.77	320.45	9.412
619	620			0	0 0.0	00 RG 00 MF	79.82 1.63		35 105	2,794 171	0.00405 Vs/p 0.00405 Vs/p		11.32 0.69													
	F	Residential			0	JU IVIF	81.45			2,965		3.45	12.01	41.40	16.29	57.69	66.77									
	_					76 Com/Ind School	489.68 0.00				0.03500 Vs/ha 0.20000 Vs/ha		17.1													
		Non-Residential				OCHOO	489.68				0.20000 V3/1la	2.78	17.1	47.72	97.94	145.65	168.58			6	19 Ups MH	620 Dns MH				
	1	Direct Flow														153.18	177.30									
		Fotal			29.	76	571.13										412.65	460.17 Min	675 0.0030 0.0010		45 310.31	140 309.89 0.420	0.05 n	9.46	319.86	9.292
620	621			0	0 -		79.82		35	2,794	0.00405 l/s/p		11.32					Min	. 0.0010							
		Residential				MF	1.63 81.45	1	105	171 2,965	0.00405 Vs/p	3.45	0.69 12.01	41.40	16 20	57.69	66.77									
	_	residential			-	Com/Ind	489.68			2,900	0.03500 l/s/ha	3.43	17.1	41.40	10.29	37.09	00.77									
621 is the lift sta	tion	Non-Residential			-	School	0.00 489.68				0.20000 l/s/ha	2.78	17.1	47.72	97.94	145.65	168.58			6	20 Ups MH	621 Dns MH				
		Direct Flow					100.00					2.70				153.18	177.30					02.1 D.10 MII				
		Fotal					571.13							89.12		356.53	412.65	531.36	675 0.0040		86 309.84	50 309.64 0.200	0.05 n	9.34	320.15	9.832
											<u> </u>		DW	/E	WWF			Min	. 0.0010				·			
													low	242.30 l/s	VV VV F	356.53 l/s					2	883 m of Pipe		Depth		
			Total Area	a:	571.	13 ha	LPS	14.	.56 l/s			600 17	mm FM DR			0.220 sq.m 1.62 m/s	1					306.64 Approx. Min.	Pump Elevation	3.59 n 9.83 n		
																								"		

Phase 2 Density: units per ha I/I = 0.2 I/s/ha 2320-002 10 pph

						Population				Flow					System Design		UPS	Pipe Data		DNS	UPS	DNS		
Manhole / Basin Ups Dns		Sag Accun Manhole Sags	n. Are (ha		Accum.	Density (people/ha)	Accum. (people)	Sewage Generation (rate)	PF	Dry Weath Peak		l-Area Te /s) (l/		esign /s)		meter Slope n) (m/m)	Rim	Ups Inv Le	ngth Dns Inv Pipe Drop		Curved Depth	Rim		epth
9	10	0	0	52.45 RG	52.45	3	5 1,83	36 0.00405 l/s/p		7.44														
				1.06 MF	1.06	10	15 11	11 0.00405 l/s/p		0.45														
	Residential		0		53.51		1,94		3.59	7.89	28.35	10.70	39.06	45.20										
				- Com/Ind 4.51 School	0.00 4.51			0.20000 l/s/ha 0.20000 l/s/ha		0.9						375			12 Upstream Stub					
	Non-Residential			4.51 School	4.51			0.20000 l/s/na	5.00		1 51	0.90	5.41	6.26		3/5		9 Ups MH	12 Upstream Stub 10 Dns MH					
	Direct Flow 7 Q LPS				4.31				5.00	29.12	4.31	0.50	29.12	33.70				9 Ops Will	TO DIS WIT					
	CP			58.02	58.02					46.70	32.86	11.60	73.59	85.17	85.85 Min	375 0.0024 0.0015			72.66 314.83 0.174 cludes 12m 375mm stub upstre		n 7	<mark>7.17</mark> 3	21.04	5.839
10	11	0	0	3.54 RG	55.99	3	5 1,96	60 0.00405 l/s/p		7.94					IVIII I	. 0.0015			ed on lateral connection require					
			-	1.23 MF	2.29		15 24	40 0.00405 l/s/p		0.97									7					
	Residential		0		58.28		2,20	00	3.55	8.91	31.67	11.66	43.32	50.14										
				 Com/Ind 				0.20000 l/s/ha		-														
	No. Beetleved			2.00 School	6.51			0.20000 l/s/ha	5.00	1.3	0.54	4.00	7.04	0.04				40.11 141.1	44.5181					
	Non-Residential Direct Flow				6.51				5.00	1.3	6.51	1.30	7.81 29.12	9.04 33.70				10 Ups MH	11 Dns MH					
	Total			6.77	64.79					20.43	38.18	12.96	80.25	92.89	98.71	450 0.0012		1.04 314.75	84.07 314.65 0.10	0.05	n 5	5 <mark>.84</mark> 3	21.38	6.280
11	12	0	0	- RG	55.99	3	5 1,96	60 0.00405 l/s/p		7.94					Min	. 0.0012								
- 11	12	U	U	- KG - MF	2.29			10 0.00405 l/s/p		0.97														
	Residential		0		58.28		2,20		3.55		31.67	11.66	43.32	50.14										
				 Com/Ind 	0.00			0.20000 l/s/ha		-														
				 School 	6.51			0.20000 l/s/ha		1.3								6.78						
	Non-Residential Direct Flow				6.51				5.00	1.3	6.51	1.30	7.81 29.12	9.04 33.70				11 Ups MH	12 Dns MH					
	Direct Flow												29.12	33.70										
	Total			-	64.79					20.43	38.18	12.96	80.25	92.89	102.74	450 0.0013	321	1.38 314.60	84.07 314.49 0.109	0.05	n 6	<mark>3.33</mark> 3	20.97	6.030
															Min	. 0.0012								
12	13	0	0	- RG - MF	55.99	3	5 1,96	0.00405 l/s/p		7.94														
	Residential		0	- MF	2.29 58.28		15 24 2,20		3.55	0.97 8.91	24.07	11.66	43.32	50.14										
	Residential		U	- Com/Ind			2,20	0.20000 l/s/ha	3.55	6.91	31.0/	11.00	43.32	50.14										
				- School	6.51			0.20000 l/s/ha		1.3								6.53						
	Non-Residential				6.51				5.00	1.3	6.51	1.30	7.81	9.04				12 Ups MH	13 Dns MH					
	Direct Flow												29.12	33.70										
	Total			_	64.79					20.43	38.18	12.96	80.25	92.89	106.62	450 0.0014	320	0.97 314.44	98 314.30 0.13	0.05	n 6	. OB 3	21.03	6 277
	Total				04.73					20.40	30.10	12.30	00.20	32.03	Min		320	J.37 J14.44	30 314.30 0.13	0.03		,.00 3	21.00	0.211
13	14	0	0	12.97 RG	68.96	3	5 2,41	14 0.00405 l/s/p		9.78					IVIII I	. 0.0012								
				- MF	2.29	10	15 24	40 0.00405 l/s/p		0.97														
	Residential		0		71.25		2,65		3.49	10.75	37.49	14.25	51.74	59.88										
				- Com/Ind				0.20000 l/s/ha		-														
	Non-Residential			2.51 School	9.02 9.02			0.20000 l/s/ha	5.00	1.8	9.02	1.80	10.82	12.53				13 Ups MH	14 Dns MH					
	Direct Flow				3.02	•			3.00	1.0	3.02	1.00	29.12	33.70				15 Opa Wil 1	14 DIS WIT					
	Total			15.48	80.27					25.11	46.51	16.05	91.68	106.12	110.36 Min	450 0.0015 0.0012		1.03 314.25	98 314.11 0.14	0.05	n (3.33 3	20.93	6.374
14	15	0	0	16.94 RG	85.90	3	5 3,00	0.00405 l/s/p		12.18					Min	. 0.0012								
17		· ·	U	- MF	2.29		15 3,00			0.97														
	Residential		0		88.19		3,24	47	3.41		44.89	17.64	62.53	72.37										
				 Com/Ind 				0.20000 l/s/ha		-														
				- School	9.02			0.20000 l/s/ha		1.8														
	Non-Residential Direct Flow				9.02				5.00	1.8	9.02	1.80	10.82 29.12	12.53 33.70				14 Ups MH	15 Dns MH					
	Direct Flow												29.12	33.70										
	Total			16.94	97.21					29.91	53.91	19.44	102.47	118.60	120.90 Min	450 0.0018 0.0012		0.93 314.06	98 313.88 0.176	0.05	n 6	6.42 3	20.82	6.490

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Phase 2

2320-002 10 pph Population Density Flow Dry Weath(Peal System Design Capacity Diameter Slope Pipe Data
Ups Inv Length DNS Manhole / Basin Total Accum. Area Zoning Accum. Accum. Sewage Generation I/I -Area Design Capacity Ups Dns Sags (ha) Area (people/ha) (people) (rate) (l/s) (l/s) (l/s) (Vs) (l/s) (mm) (m/m) (l/s) 15 3.89 RG 89.78 35 3,142 0.00405 l/s/p 12.73 2.29 105 0.00405 l/s/p 0.97 Residential 92.07 3.383 13.70 46.56 18.41 64.97 75.20 0.00 9.02 0.20000 l/s/ha Com/Ind School 0.20000 l/s/ha 1.8 Non-Residential 12.53 15 Ups MH 16 Dns MH Direct Flow 29 12 33.70 313.64 0.186 3.89 101.09 31.02 55.58 20.22 104.92 121.43 124.21 450 0.0019 0.0012 16 17 RG 89.78 35 3,142 0.00405 l/s/p 12.73 MF 2.29 105 240 3,383 0.00405 l/s/p 0.97 Residential 13.70 46.56 18.41 64.97 75.20 0.20000 I/s/ha Com/Ind 0.00 9.02 1.8 0.20000 l/s/ha School Non-Residential 9.02 10.82 12.53 16 Ups MH 17 Dns MH Direct Flow 33.70 313.40 0.196 101.09 31.02 55.58 20.22 104.92 121.43 127.44 450 0.0020 321.06 7.212 17 18 0 10.56 RG 100 34 3 512 0.00405 l/s/n 14 23 35 105 0.00405 l/s/p 2.29 240 0.97 102.63 3,752 15.20 51.04 20.53 71.57 82.84 17 is at the quarter line 0.00 9.02 0.20000 l/s/ha Com/Ind 1.8 0.20000 l/s/ha School Non-Residential 10.82 12.53 17 Ups MH 18 Dns MH Direct Flow 10.56 111.65 34.01 60.06 22.33 111.52 129.07 130.58 450 0.0021 313.14 0.206 319.19 5.598 18 19 11.03 RG 15.79 0.97 111.38 35 105 3.898 0.00405 l/s/p 240 0.00405 l/s/p 2.29 Residential 16.77 55.66 22.73 78.39 90.73 Com/Ind 0.20000 l/s/ha 1.8 School 9.02 0.20000 l/s/ha Non-Residential 9.02 10.82 12.53 18 Ups MH 19 Dns MH 1.8 Direct Flow 33.70 11.03 122.69 37.14 64.68 24.54 118.34 136.97 139.60 450 0.0024 319.19 313.09 312.86 0.235 319.43 6.123 19 3.85 RG 20 115 22 35 4 033 0.00405 l/s/n 16 34 0.19 MF 105 0.00405 l/s/p 2.48 260 1.05 Residential 117.70 17.39 57.49 23.54 81.03 93.78 0.20000 l/s/ha 0.20000 l/s/ha 0.00 9.02 Com/Ind 1.8 School 12.53 19 Ups MH 20 Dns MH Non-Residential 10.82 Direct Flow 4.04 126.72 38.39 66.51 25.34 120.97 140.02 450 0.0025 312.56 0.246 319.20 0.0012 20 0.00405 l/s/p 21 RG MF 115.22 35 105 4.033 16.34 2.48 260 0.00405 l/s/p 1.05 117.70 4,293 17.39 57.49 23.54 81.03 93.78 Residential Com/Ind 0.20000 l/s/ha 0.20000 l/s/ha School 9.02 Non-Residential 9.02 10.82 12.53 20 Ups MH 21 Dns MH Direct Flow 33.70 126.72 38.39 66.51 25.34 120.97 140.02 145.30 450 0.0026 319.20 312.51 312.30 0.208 0.05 318.40 5.647

I/I = 0.2 I/s/ha

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LaCrete North Trunk Sewer Phase 2

Density:

2320-002 10 pph
 Pipe Data
 DNS
 UPS

 Ups Inv
 Length
 Dns Inv
 Pipe Drop
 MH Drop
 Curved
 Depth
 Population Density Flow Dry Weath(Peak System Design Capacity Diameter Slope Manhole / Basin Sag Accum. Area Zoning Sewage Generation I/I -Area Total Design Accum. Accum. Ups Dns Manhole Sags (ha) Area (people/ha) (people) (rate) (l/s) (l/s) (l/s) (l/s) (Vs) (l/s) (mm) (m/m) 21 22 RG 115.22 35 4,033 0.00405 l/s/p 16.34 2.48 105 0.00405 l/s/p 1.05 260 Residential 117.70 4,293 3.31 17.39 57.49 23.54 81.03 93.78 Com/Ind 0.00 9.02 0.20000 l/s/ha 0.20000 l/s/ha 1.8 School Non-Residential 10.82 12.53 21 Ups MH 22 Dns MH Direct Flow 29.12 33.70 126.72 38.39 66.51 25.34 120.97 140.02 148.07 450 0.0027 318.00 5.513 0.0012 22 23 0 5.16 RG 120.38 35 4,213 0.00405 l/s/p 17.07 260 4,474 2.48 105 0.00405 l/s/p 1.05 Residential 122.86 18.12 59.61 24.57 84.18 97.44 0.20000 l/s/ha Com/Ind 0.00 9.02 0.20000 l/s/ha 1.8 School Non-Residential 9.02 10.82 12.53 22 Ups MH 23 Dns MH Direct Flow 33.70 143.67 450 0.0028 311.71 0.280 5.16 131.88 39.85 68.63 26.38 124.13 150.79 317.50 5.343 23 24 0 22.89 RG 143 27 0.00405 I/s/n 20.31 35 5.014 24 is Lift Station 2 0.47 MF 2.95 105 309 0.00405 l/s/p 1.25 69.44 29.24 98.68 Residential 146.22 5,324 21.57 114.21 0.00 9.02 0.20000 l/s/ha 0.20000 l/s/ha Com/Ind 1.8 School Non-Residential 10.82 12.53 23 Ups MH 24 Dns MH Direct Flow 311.56 0.100 23.36 155.24 23.37 78.46 31.05 138.62 160.44 201.50 450 0.0050 317.70 5.693 DWF Flow 138.62 l/s 1317 m of Pipe in estimate (upstream stub) Depth Total Area: 155.24 ha LPS Flow: 29.12 Vs 400 mm FM 11 DR 0.084 sq.m 1305 m of Trunk 5.39 min 308.56 Approx. Min. Pump Elevation 1.65 m/s 7.26 max

I/I = 0.2 I/s/ha

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Mckenzie County Sanitary Sewer Flows LaCrete North Trunk Sewer

Phase 3 No 2320-002	orth Expansion					Density: units per ha		10	350 Vp/d Res 3,000 Vha/d Comme			I/I = 0.2	l/s/ha l/s/sag														
Manhole / Basin Ups Dns		Sag Manhole	Accum. Sags	Area (ha)	Zoning	Accum.	Population Density (people/ha)	Accum. (people)	Sewage Generation (rate)	PF	Flow Dry Weath Peak [/s) (l/s)	I/I -Area (I/s)	Total (l/s)	Desig (l/s)	gn		Diameter	Slope (m/m)	UPS Rim	Pipe Data Ups Inv		Dns Inv	Pipe Drop MH		UPS Curved Depth	DNS Rim	Depth
902	903 West		0	0 -	SF MF	0.00		35	- 0.00405 Vs/p - 0.00405 Vs/p		0.00																
	Residential				00 Com/Ind	0.00 120.00			0.03500 Vs/ha	2.50	4.2	0.00 -		0.00	0.00												
	Non-Residential Direct Flow				School	0.00 120.00			0.20000 l/s/ha	5.00	4.2	21.00 24.00		45.00 0.00	52.08 0.00					902 Ups MH		900	Dns MH				
	Total			120.0	00	120.00									52.08	53.81			322	2.28 318.51			2.010	0.075	n 3.	47 32	1.45 4.650
903	904 Residential		0	0 -	RG MF	0.00 0.00 0.00		35 ·	- 0.00405 Vs/p - 0.00405 Vs/p	2.50	0.00 0.00	0.00 -		0.00	0.00		Min.	0.0022		Invert set I	based on la	iteral assess	ment.				
	Non-Residential Direct Flow				O Com/Ind School	180.00 0.00 180.00			0.03500 Vs/ha 0.20000 Vs/ha	4.37	6.3	27.52 36.00		63.52	73.52 0.00					903 Ups MH		904	Dns MH				
	Total			60.0	00	180.00					•			0.00	73.52	74.35			32	1.45 316.43	40	0 315.6	0.820	0.05	n 4.	65 32	1.14 5.160
904	905		0		00) RG MF	0.00 0.00		105 -	(0) 0.00405 Vs/p - 0.00405 Vs/p		0.00 0.00						Min.	0.0015									
	Residential				00 Com/Ind School	0.00 240.00 0.00			0.03500 Vs/ha 0.20000 Vs/ha	2.50	8.4	0.00 (0.00)		0.00	0.00												
	Non-Residential Direct Flow				_	240.00				3.84	8.4	32.24 48.00		80.24 0.00	0.00					904 Ups MH			Dns MH				
905	Total 906		0	0 -	RG	240.00		35	(0) 0.00405 Vs/p		0.00				92.87	94.37	Min.	0.0029	32	1.14 315.56	i 40	0 314.30	1.260	0.075	n 5.:	<u>21</u> 32	0.42 5.750
	Residential				MF 00 Com/Ind	0.00 0.00 270.00			0.00405 Vs/p (0) 0.03500 Vs/ha	2.50	0.00 (0.00) 9.5	0.00 (0.00)		0.00	0.00												
	Non-Residential Direct Flow				School	0.00 270.00			0.20000 Vs/ha	3.64	9.5	34.39 54.00		88.39 0.00	102.31 0.00					905 Ups MH		906	Dns MH				
906	Total 613		0	0 0.0	00 00 RG	270.00		35	0 0.00405 Vs/p		0.00				102.31	102.74	450 Min.	0.0013	320	0.42 314.22	40	0 313.60	0.620	0.05	n 5.	<mark>75</mark> 32	0.56 6.510
900	Residential			0.0	00 MF	0.00 0.00 0.01 300.00		105	0 0.00405 Vs/p 0 0.00405 Vs/p 0 0.03500 Vs/ha	4.48	0.00	0.01 0.00		0.01	0.01												
	Non-Residential Direct Flow				School	0.00 300.00			0.20000 Vs/ha	3.47		36.45 60.00		96.45 0.00	111.63					906 Ups MH		613	Dns MH				
	Total			30.0)1	300.01								96.46	111.64	113.98	450	0.0016	320	0.56 313.55	40	0 312.81	0.740	0.05	n 6.	5 <mark>6</mark> 32	0.66 7.400
				ea: 300.0 /hen this tru		alone, the contri	buting area	s 246ha and th	e peak wet weather flow is 8	4.1 l/s								Conr	nect to Tr	unk 312.81		0 m of Pipe				47 m 40 m	

2320-002 - North Expansion Future Trunk - Reduced Flows - 11/16/2020



Appendix B

Detailed Cost Estimate

PHASE 1 - CONSTRUCTION COST ESTIMATES

ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
A1.	Safety flag persons, barricades, permits, eco plan	\$10,000.00	l.s.	1	\$10,000.0
A.2 Crop damage reimbursement A3. Hydrovac/locate existing shallow conflict utilities, gas mai A4. Clearing & grubbing	Crop damage reimbursement	\$2.00	s.m	86,550	\$173,100.0
	Hydrovac/locate existing shallow conflict utilities, gas mains	\$10,000.00	l.s.	1	\$10,000.0
A4.	Clearing & grubbing	\$7,500.00	ha.	1	\$7,500.0
A5.	Topsoil stripping of proposed construction limits & laydown areas (push to side of R/W)	\$3.50	c.m.	14,425	\$50,487.5
A6.	Topsoil restoration of construction R.O.W. & laydown areas (restore to existing)	\$3.50	c.m.	14,425	\$50,487.5
	Restoration of existing gravel access road/road allowance c/w				
A7.	cloth/grid, 400mm GBC	\$45.00	s.m.	250	\$11,250.0
A8.	Restoration of existing gravel access driveway c/w cloth/grid, 300mm GBC	\$35.00	s.m.	0	\$0.0
A9.	Restoration of existing Pavement c/w cloth/grid, 600mm GBC, 120mm ACP	\$100.00	s.m.	0	\$0.0
A10.	Supply/Install 15m -600mm CSP culvert c/w tapered ends	\$5,000.00	ea	0	\$0.0
A11.	Supply/Install sanitary sewer main	400.00			40.0
	a) 250 b) 300	\$90.00 \$100.00	I.m.	0	\$0.0 \$0.0
	c) 375	\$100.00	l.m.	0	\$0.0
	d) 450	\$210.00	l.m.	395	\$82,950.0
	e) 525	\$235.00	l.m.	677	\$159,095.0
	f) 600	\$260.00	l.m.	915	\$237,900.0
	g) 675	\$340.00	l.m.	898	\$305,320.0
	h) 750	\$420.00	l.m.	0	\$0.0
A12.	Trenching/Backfilling				
	a) 0- 4 m depth of bury	\$90.00	l.m.	259	\$23,310.0
	b) 4-5m depth of bury	\$130.00		272	\$35,360.0
	c) 5- 6m depth of bury	\$180.00	l.m.	1,321	\$237,690.0
	d) 6- 7m depth of bury e) 7- 8 m depth of bury	\$250.00 \$390.00	I.m.	134 320	\$33,375.0 \$124,800.0
	f) 8-9m depth of bury	\$640.00	l.m.	300	\$192,000.0
	g) 9-10m depth of bury	\$800.00	l.m.	278	\$222,400.0
	h) 10-11m depth of bury	\$900.00	l.m.	0	\$0.0
	i) 11-12m depth of bury	\$1,100.00	l.m.	0	\$0.0
A13.	Supply/Install SR concrete manholes c/w frame & covers for 19 units				
	a) 1200mm SR Precast base	\$3,500.00	ea	11	\$38,500.0
	b) 1500mm SR Precast base (>600 pipe)	\$9,500.00	ea	16	\$152,000.0
	c) Supply install 1200mm concrete barrels c/w rings & F.C	\$2,200.00 \$3,600.00	v.m.	48.5	\$106,630.8
	d) Supply install 1500mm concrete barrels c/w rings & F.C		v.m.	110	\$396,981.1
A15. Supply/Install aluminum safety platform		\$1,850.00	ea.	0	\$0.0

PHASE 1 - CONSTRUCTION COST ESTIMATES

ITEN	1 DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
A1	7. Video Inspection	\$17.00	l.m.	2,885	\$49,045.00
A1	8. Lift station	\$1,900,000.00	ea.	1.00	\$1,900,000.00
A1	9 Forcemain				
	200mm HDPE DR11 Forcemain	\$325.00	l.m	0	\$0.00
	300mm HDPE DR11 Forcemain	\$390.00	l.m	0	\$0.00
	400mm HDPE DR11 Forcemain	\$500.00	l.m	0	\$0.00
	450mm HDPE DR11 Forcemain	\$550.00	l.m	0	\$0.00
	500mm HDPE DR11 Forcemain	\$655.00	l.m	0	\$0.00
	550mm HDPE DR11 Forcemain	\$710.00	l.m	0	\$0.00
	600mm HDPE DR11 Forcemain	\$750.00	l.m	50	\$37,500.00
	850mm HDPE DR11 Forcemain	\$1,000.00	l.m		\$0.00
	Auger/Receiving Pits	\$10,000.00	ea.	0	\$0.00
	Trenching 4- 5m depth of bury	\$130.00	l.m.	50	\$6,500.00
	Replace ex. Inlet MH at Lagoon	\$150,000.00	ea.	1	\$150,000.00
	Air Relief Chambers	\$30,000.00	ea.	0	\$0.00
	TOTAL				\$4,846,181.99

	Phase 1
Gravity	\$2,753,000.00
LS/FM	\$2,094,000.00
Subtotal	\$4,847,000.00
Contingency	\$970,000.00
Engineering	\$485,000.00
Total	\$6,302,000.00

Phase 1

Lined M	anhole Co	sts		
MH	D	epth Per/m	Amou	nt
	595	4.2	1,600	6,720
	596	4.0	1,600	6,462
	597	4.7	1,600	7,441

12.9

Subtotal	\$21,000
Contingency (20%)	\$4,000
Engineering (10%)	\$2,000
Total	\$27,000

PHASE 2 - CONSTRUCTION COST ESTIMATES

ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
A1.	Safety flag persons, barricades, permits, eco plan	\$10,000.00	l.s.	1	\$10,000.00
A.2	Crop damage reimbursement	\$2.00	s.m	26,580	\$53,160.00
۸2	Hudrovae/locate existing shallow conflict utilities gas mains	\$10,000.00	l c	1	\$10,000,00
A3.	Hydrovac/locate existing shallow conflict utilities, gas mains		l.s.	2.0	\$10,000.00
A4.	Clearing & grubbing	\$7,500.00	ha.	2.0	\$15,000.00
A5.	Topsoil stripping of proposed construction limits & laydown areas (push to side of R/W)	\$3.50	c.m.	10,000	\$35,000.00
A6.	Topsoil restoration of construction R.O.W. & laydown areas (restore to existing)	\$3.50	c.m.	10,000	\$35,000.00
A7.	Restoration of existing gravel access road/road allowance c/w cloth/grid, 400mm GBC	\$45.00	s.m.	200	\$9,000.00
A8.	Restoration of existing gravel access driveway c/w cloth/grid, 300mm GBC	\$35.00	s.m.	0	\$0.00
	Restoration of existing Pavement c/w cloth/grid, 600mm GBC,				
A9.	120mm ACP	\$100.00	s.m.	200	\$20,000.00
A10.	Supply/Install 15m -600mm CSP culvert c/w tapered ends	\$5,000.00	ea	0	\$0.00
A11.	Supply/Install sanitary sewer main a) 250	\$90.00	l.m.	0	\$0.00
	b) 300	\$100.00	l.m.	0	\$0.00
	c) 375	\$125.00	l.m.	97	\$12,082.50
	d) 450	\$210.00	l.m.	1,232	\$258,791.40
	e) 525	\$235.00	l.m.	0	\$0.00
	f) 600	\$260.00	l.m.	0	\$0.00
	g) 675	\$340.00	l.m.	0	\$0.00
	h) 750	\$420.00	l.m.	0	\$0.00
A12.	Trenching/Backfilling a) 0- 4 m depth of bury	\$90.00	l m	0	\$0.00
	b) 4-5m depth of bury	\$130.00	l.m.	0	\$0.00
	c) 5- 6m depth of bury	\$180.00	l.m.	378	\$68,040.00
	d) 6-7m depth of bury	\$250.00	l.m.	927	\$231,750.00
	e) 7-8 m depth of bury	\$390.00	l.m.	12	\$4,680.00
	f) 8- 9m depth of bury	\$640.00	l.m.	0	\$0.00
	g) 9-10m depth of bury	\$800.00	l.m.	0	\$0.00
	h) 10-11m depth of bury	\$900.00	l.m.		
	i) 11-12m depth of bury	\$1,100.00	l.m.		
A13.	Supply/Install SR concrete manholes c/w frame & covers for 19 units				
	a) 1200mm SR Precast base	\$3,500.00	ea	16.0	\$56,000.00
	b) 1500mm SR Precast base (>600 pipe)	\$9,500.00	ea	0.0	\$0.00
	c) Supply install 1200mm concrete barrels c/w rings & F.C	\$2,200.00	v.m.	78.9	\$173,688.72
	d) Supply install 1500mm concrete barrels c/w rings & F.C	\$3,600.00	v.m.	0.0	\$0.00
	Supply/Install aluminum safety platform	\$1,850.00	ea.	0.0	\$0.00

PHASE 2 - CONSTRUCTION COST ESTIMATES

ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
A16	Base stabilizing material (screened rock)	\$70.00	c.m.	600	\$42,000.00
A17	. Video Inspection	\$17.00	l.m.	1,329	\$22,593.00
A18	Lift station	\$1,700,000.00	ea.		\$1,700,000.00
A19	Forcemain				
	200mm HDPE DR11 Forcemain	\$325.00	l.m	0	\$0.00
	300mm HDPE DR11 Forcemain	\$390.00	l.m	0	\$0.00
	400mm HDPE DR11 Forcemain	\$500.00	l.m	2223	\$1,111,500.00
	450mm HDPE DR11 Forcemain	\$550.00	l.m	0	\$0.00
	500mm HDPE DR11 Forcemain	\$655.00	l.m	0	\$0.00
	550mm HDPE DR11 Forcemain	\$710.00	l.m	0	\$0.00
	600mm HDPE DR11 Forcemain	\$750.00	l.m	0	\$0.00
	850mm HDPE DR11 Forcemain	\$1,000.00	l.m	0	\$0.00
	Auger/Receiving Pits	\$10,000.00	ea.	4	\$40,000.00
	Trenching 4- 5m depth of bury	\$130.00	l.m.	0	\$0.00
	Modifications to Phase 1 Lift Station	\$260,000.00	l.s.	1	\$260,000.00
	Air Relief Chambers	\$30,000.00	ea.	2	\$60,000.00
	TOTAL				\$4,228,285.62

	Phase 2
Gravity	\$1,057,000.00
LS/FM	\$3,172,000.00
Subtotal	\$4,229,000.00
Contingency 20%	\$846,000.00
Engineering 10%	\$423,000.00
Total	\$5,498,000.00

Phase 2 Lined Manhole Costs

MH		Depth	Per/m	Amour	nt
	9	7.5		1,600	12,064
	10	6.3		1,600	10,063
	11	6.8		1,600	10,848

20.6

Subtotal	\$33,000
Contingency (20%)	\$7,000
Engineering (10%)	\$3,000
Total	\$43,000

NORTH SANITARY SEWER EXPANSION PHASE 3 - CONSTRUCTION COST ESTIMATES

ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
A1.	Safety flag persons, barricades, permits, eco plan	\$10,000.00	l.s.	1	\$10,000.00
A.2	Crop damage reimbursement	\$2.00	s.m	28600	\$57,200.00
A3.	Hydrovac/locate existing shallow conflict utilities, gas mains	\$10,000.00	l.s.	1	\$10,000.00
A4.	Clearing & grubbing	\$7,500.00	ha.	1.0	\$7,500.00
A5.	Topsoil stripping of proposed construction limits & laydown areas (push to side of R/W)	\$3.50	c.m.	11,000	\$38,500.00
A6.	Topsoil restoration of construction R.O.W. & laydown areas (restore to existing)	\$3.50	c.m.	11,000	\$38,500.00
	Restoration of existing gravel access road/road allowance c/w				
A7.	cloth/grid, 400mm GBC	\$45.00	s.m.	0	\$0.00
A8.	Restoration of existing gravel access driveway c/w cloth/grid, 300mm GBC	\$35.00	s.m.	0	\$0.00
A9.	Restoration of existing Highway c/w cloth/grid, 600mm GBC, 120mm ACP	\$100.00	s.m.	0	\$0.00
A10.	Supply/Install 15m -600mm CSP culvert c/w tapered ends	\$5,000.00	ea	0	\$0.00
A11.	Supply/Install sanitary sewer main				
	a) 250	\$90.00	l.m.	0	\$0.00
	b) 300	\$100.00	l.m.	600	\$60,000.00
	c) 375	\$125.00	l.m.	800	\$100,000.00
	d) 450	\$210.00	l.m.	800	\$168,000.00
	e) 525	\$235.00	l.m.	0	\$0.00
	f) 600	\$260.00	l.m.	0	\$0.00
	g) 675	\$340.00	l.m.	0	\$0.00
	h) 750	\$420.00	l.m.	0	\$0.00
A12.	Trenching/Backfilling				
	a) 0- 4 m depth of bury	\$90.00	l.m.	0	\$0.00
	b) 4- 5m depth of bury	\$130.00	l.m.	1,000	\$130,000.00
	c) 5- 6m depth of bury	\$180.00	l.m.	400	\$72,000.00
	d) 6- 7m depth of bury e) 7- 8 m depth of bury	\$250.00 \$390.00	l.m.	800	\$200,000.00 \$0.00
	f) 8-9m depth of bury	\$640.00	l.m.	0	\$0.00
	g) 9-10m depth of bury	\$800.00	I.m.	0	\$0.00
	h) 10-11m depth of bury	\$900.00	l.m.	0	\$0.00
	i) 11-12m depth of bury	\$1,100.00	l.m.	0	\$0.00
A13.	Supply/Install SR concrete manholes c/w frame & covers for 19 units				
	a) 1200mm SR Precast base	\$3,500.00	ea	1.0	\$3,500.00
	b) 1500mm SR Precast base	\$9,500.00	ea	5.0	\$47,500.00
	c) Supply install 1200mm concrete barrels c/w rings & F.C	\$2,200.00	v.m.	1.7	\$3,784.00
	d) Supply install 1500mm concrete barrels c/w rings & F.C	\$3,600.00	v.m.	30.0	\$108,000.00
A15.	Supply/Install aluminum safety platform	\$1,850.00	ea.	0.0	\$0.00
A16.	Base stabilizing material (screened rock)	\$70.00	c.m.	600	\$42,000.00
A17.	Video Inspection	\$17.00	l.m.	2,200	\$37,400.00
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PHASE 3 - CONSTRUCTION COST ESTIMATES

ITEM	DESCRIPTION	UNIT PRICE	UNIT	QUANTITY	AMOUNT
		4			
A18.	Lift station	\$2,000,000.00	ea.	0.00	\$0.00
A19	Forcemain				_
	200mm HDPE DR11 Forcemain	\$125.00	l.m	0	\$0.00
	300mm HDPE DR11 Forcemain	\$225.00	l.m	0	\$0.00
	350mm HDPE DR11 Forcemain	\$260.00	l.m	0	\$0.00
	500mm HDPE DR11 Forcemain	\$300.00	l.m	0	\$0.00
	Auger/Receiving Pits	\$10,000.00	ea.	0	\$0.00
	Trenching 4- 5m depth of bury	\$130.00	l.m.	0	\$0.00
	Modifications at the Lagoon Lift Station	\$120,000.00	l.s.	1	\$120,000.00
	Air Relief Chambers	\$30,000.00	ea.	0	\$0.00
	TOTAL				\$1,253,884.00

	Phase 3
Gravity	\$1,254,000.00
Contingency 30%	\$377,000.00
Engineering 10%	\$126,000.00
Total	\$1,757,000.00